

Study of Strength Development in Reinforced Concrete Beams Retrofitted by Different Types of Fiber Reinforced Polymers

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Abstract

In order to improve the performance of a Reinforced Cement Concrete (RCC) structure, it is better to repair or upgrade the structure by retrofitting. This study revealed the results of a nonlinear finite element analysis conducted on reinforced cement concrete beams strengthened with Glass Fiber Reinforced Polymer (GFRP) laminates, Carbon Fiber Reinforced Polymer (CFRP) laminates and Aramid Fiber Reinforced Polymer (AFRP) laminates. Five beams with different fiber length and orientation for each fiber retrofitted beams were cast and analyzed for the present study. All the beams were tested under four-point bending till failure. The results obtained through non linear Finite Element Method (FEM) analysis using ANSYS14.5 software can be used to study how different parameters affect retrofitted beam behavior and investigate how Externally Bounded Reinforcement (EBR) should be applied in order to obtain the maximum load carrying capacity.

Keywords: *Glass Fiber Reinforced Polymer, Carbon Fiber Reinforced Polymer, Aramid Fiber Reinforced Polymer, Finite element analysis.*

1. Introduction

Reinforced concrete (RC) structures often have to face modification and improvement of their performance during their service life. In such circumstances there are two possible solutions: replacement or retrofitting. Full structural replacement might have determinate disadvantages such as high costs for material and labour, a stronger environmental impact and inconvenience due to interruption of the function of the structure, e.g. traffic problems. When possible, it is often better to repair or upgrade the structure by retrofitting in the last decade, the development of strong epoxy glue has led to the growth of externally bonded reinforcement (EBR) strengthening techniques which has great potential in the field of upgrading structures. Basically the technique involves gluing steel plates or (fiber reinforced polymer) FRP laminates to the surface

of concrete. FRP can be convenient compared to steel for a number of reasons. These materials have higher ultimate strength and lower density than steel. The installation is easier and temporary support until the adhesive gains its strength is not required due to the low weight. They can be formed on site into complicated shapes and can also be easily cut to length on site. Composite materials of fibers in a polymeric resin, also known as fiber-reinforced polymers (FRP), have emerged as an alternative over steel to repair, retrofit and strengthen buildings and bridges. FRP materials may offer a number of advantages over steel plates which include: light weight, noncorrosive, and exhibit high tensile strength. This work is comparative study of the behavior of concrete beams retrofitted with Carbon Fiber Reinforced Polymer (CFRP), Glass Fiber Reinforced Polymer (GFRP), and Aramid Fiber Reinforced Polymer (AFRP) with different wrapping techniques. Finite Element Analysis is used to model the behavior numerically and is used to determine the overall behavior of a structure by dividing it into number of simple elements, each of which has well defined mechanical and physical properties.

The overall aim of the present study is to investigate and improve the understanding of the behavior of reinforced concrete beams retrofitted with three different fibers-CFRP, GFRP and AFRP. Also the project aims at examining the influence of length and orientation of fibers on the behavior of RCC beams. One control beam and the five retrofitted beams each with AFRP, CFRP and GFRP laminates were loaded until failure. It is intended to show that the type, length and the orientation of the fibers strongly influence the behavior of the retrofitted beams. With the ANSYS program, the models can be used to study how different parameters affect retrofitted beam behavior and investigate how Externally Bound Reinforcement (EBR) should be applied in order to get maximum increase of load

capacity. The change in stiffness of the retrofitted beams can also be analyzed by conducting this study.

2. Experimental Analyses

2.1 FRP wrapping schemes

The RC beam specimen and the FRP wrapping schemes of 1.5 mm thickness, used for the present study has been shown schematically in the figure 1 and figure 2 respectively given below.

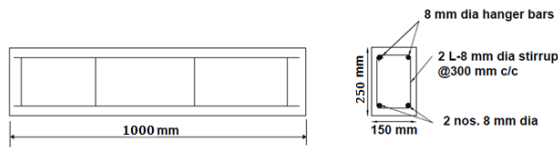


Fig 1 Reinforcement details of the beam specimen

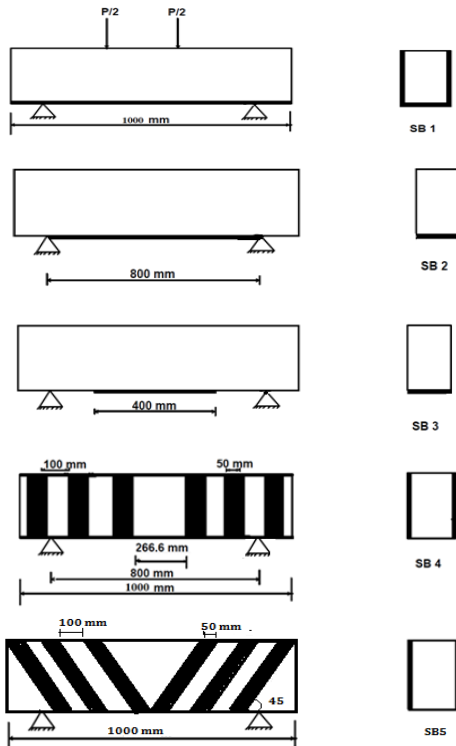


Fig 2 Different wrapping schemes

2.1.1 Strengthened in both shear and flexure

The strengthened beam designated SB1 (Strengthened Beam1) was U-wrapped, that is strengthened in both shear and flexure. For that, the wrapping was provided on two shear sides and on the bottom flexure side throughout the length.

2.1.2 Strengthened in flexure

The strengthened beam designated SB2 and SB3 was strengthened in flexure only. For that, the wrapping was provided only on the bottom flexure side of the specimen. In SB2, FRP wrapping was provided over the entire effective span (800 mm) of the specimen, while in SB3 wrapping was provided over half the effective span (400 mm) length of the specimen.

2.1.3 Strengthened in shear

The strengthened beam designated SB4 and SB5 was strengthened in shear only. For that, the wrapping was provided only on the two sides of the specimen. In SB4, 3 numbers FRP strips of 50 mm width, spaced at 100 mm center to center was placed over a distance of 266.6 mm from both the supports. In SB5, 3 numbers of FRP strips of 50 mm width, spaced at 100 mm center to center was placed at 45 degree with the longitudinal axis of the specimen.

2.2 Finite element analysis

A non-linear finite element analyses were done using ANSYS 14.5. The following are the properties assigned for the specimens.

2.2.1 Element Types

Solid65, Eight-node solid element was used to model the concrete. Steel reinforcement and steel plates were modeled using link180 element. Solid185 elements are used to model the FRP.

2.2.2 Material Properties

2.2.2.1 For M30 concrete.

Modulus of elasticity = 30000 N/mm²
Poisson's ratio = 0.2

2.2.2.2 For steel and steel plates

Modulus of elasticity = 200000 N/mm²
Poisson's ratio = 0.30
Yield stress = 415 N/mm²
Tangent modulus = 20 MPa

2.2.2.3 For FRP sheets.

Table 1 shows the material properties assigned for the FRP materials- AFRP, CFRP and GFRP laminates used for the study.

Table 1 : Material property of FRP

Properties	AFRP	CFRP	GFRP
Ex, (MPa)	13600	165000	21000
Ey, (MPa)	1482.1	9650	7000
Ez, (MPa)	1482.1	9650	7000
Gxy, (MPa)	549.13	5200	1520
Gyz, (MPa)	547	5200	2650
Gzx, (MPa)	549.13	3400	1520
NUxy	0.32	0.3	0.26
NUyz	0.35	0.3	0.3
NUzx	0.035	0.45	0.26

3. Results and discussions

The results of the ultimate load and maximum deflection of the control beam and strengthened beam specimens are tabulated below. Here SB1A, SB1C, SB1G represents Strengthened beam retrofitted with AFRP, CFRP and GFRP respectively. And similarly for other retrofitted beams.

Table 2 : Maximum deflection and Ultimate load obtained for all the specimens

Beam code	Max. Deflection (in mm)	Ultimate Load (kN)	% increase in load carrying capacity
CB	0.390	139.5	-
SB1A	0.201	188	34.2
SB1C	0.199	214	53.4
SB1G	0.216	182	30.4
SB2A	0.198	178	27.6
SB2C	0.194	199	42.6
SB2G	0.197	182	23.3
SB3A	0.188	149	7.1
SB3C	0.186	154	12.5
SB3G	0.190	147	5.4
SB4A	0.217	178	27.6
SB4C	0.215	193	38.6
SB4G	0.216	182	30.4
SB5A	0.298	184	31.9
SB5C	0.293	197	41.2
SB5G	0.306	183	31.2

The deflection was reduced in all retrofitted beams and the lowest deflection was observed in the beam strengthened with CFRP in flexure. Providing a layer of FRP composites of 1.5 mm thickness has resulted in achievement of improved load carrying capacity of the entire strengthened specimens.

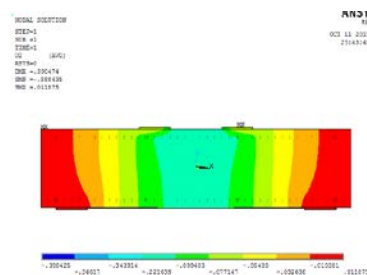


Fig 3: The displacements in the model after non-linear finite element analysis for CB

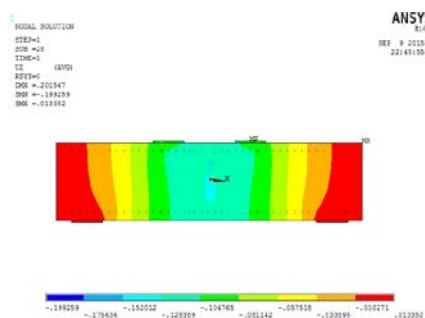


Fig 4: The displacements in the model after non-linear finite element analysis for SB1A

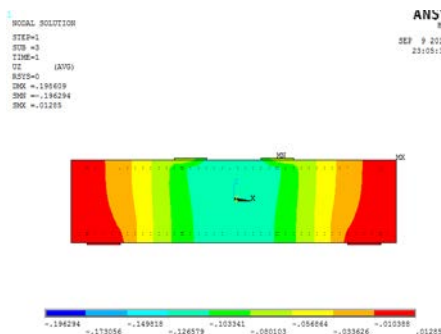


Fig 5: The displacements in the model after non-linear finite element analysis for SB2A

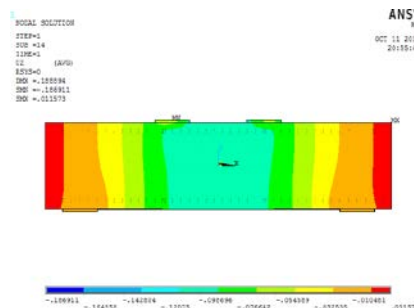


Fig 6: The displacements in the model after non-linear finite element analysis for SB3A

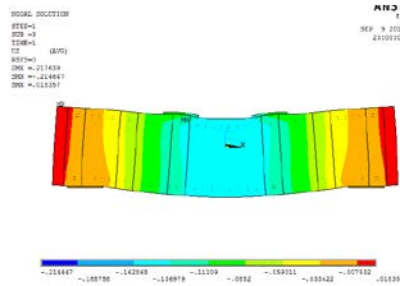


Fig 7: The displacements in the model after non-linear finite element analysis for SB4A

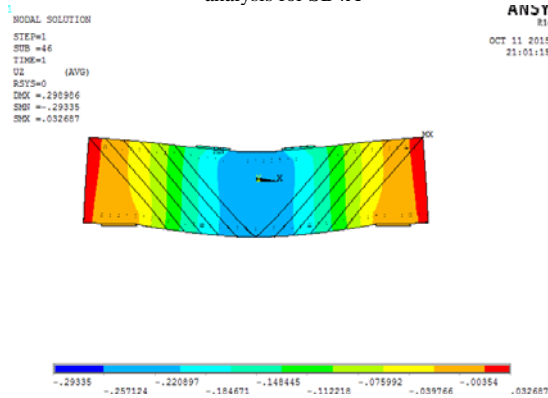


Fig 8: The displacements in the model after non-linear finite element analysis for SB5A

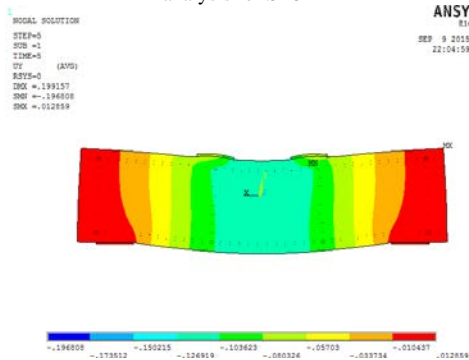


Fig 9: The displacements in the model after non-linear finite element analysis for SB1C

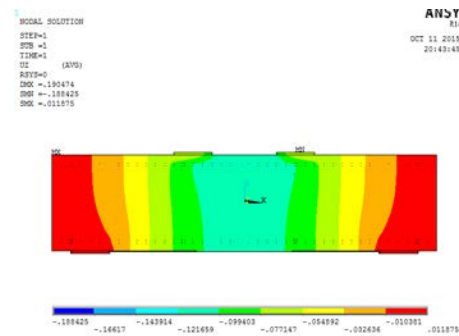


Fig 10: The displacements in the model after non-linear finite element analysis for SB2C

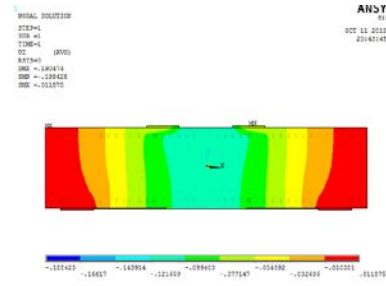


Fig 11: The displacements in the model after non-linear finite element analysis for SB3C

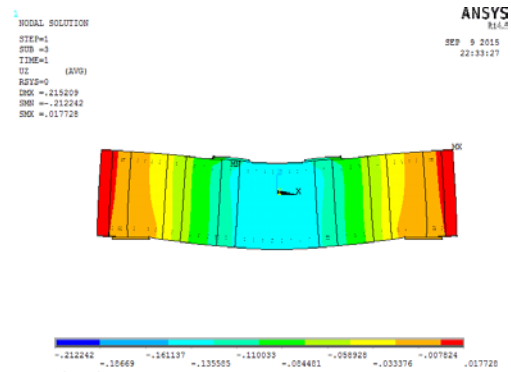


Fig 12: The displacements in the model after non-linear finite element analysis for SB4C

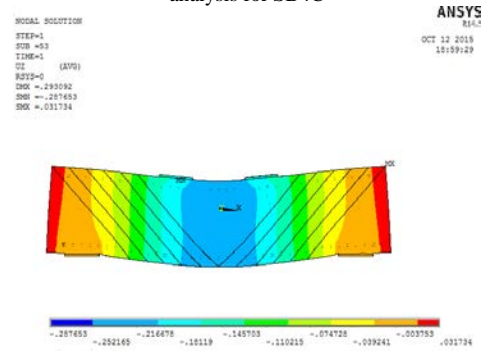


Fig 13: The displacements in the model after non-linear finite element analysis for SB5C

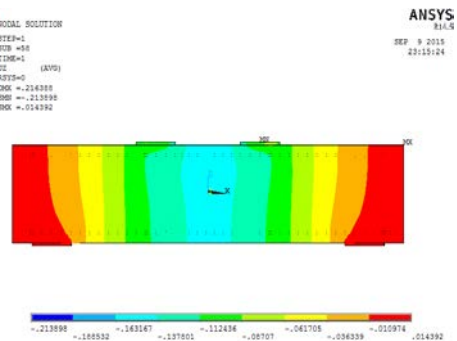


Fig 14: The displacements in the model after non-linear finite element analysis for SB1G

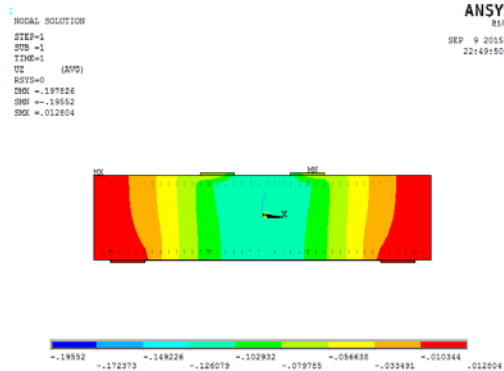


Fig 15: The displacements in the model after non-linear finite element analysis for SB2G

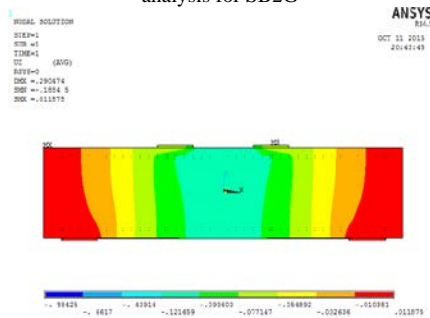


Fig 16: The displacements in the model after non-linear finite element analysis for SB3G

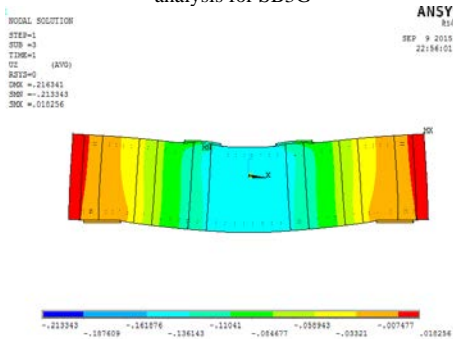


Fig 17: The displacements in the model after non-linear finite element analysis for SB4G

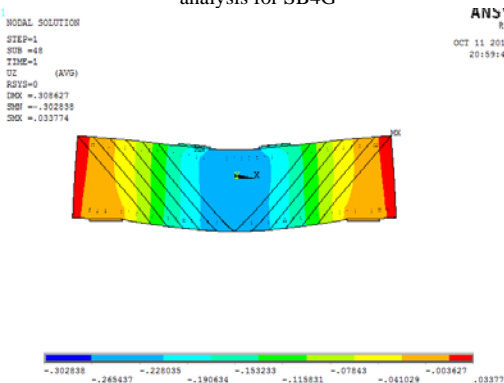


Fig 18: The displacements in the model after non-linear finite element analysis for SB5G

4. Conclusions

The main objective of the project is to study the effect of AFRP, CFRP and GFRP on behaviour of composite beams under increased loading condition and to understand the best wrapping technique among the six models of each FRP.

The following conclusions are drawn from the study

- Increase in ultimate load of about 53.4% was achieved in specimen SB1C strengthened in both flexure and shear (U wrapped with CFRP), fully wrapped in three sides. The similar pattern was observed in U wrapped AFRP and GFRP strengthened beams. From this it can be inferred that increase in area of FRP wrapping increased load carrying capacity of the concrete member..
- When wrapping was provided only on half length of the effective span, the increase in load carrying capacity was only up to 12.5% (SB3C). This means that insufficient strengthening lengths do not produce the intended strengthening effect.
- Analytical results showed that the increasing the FRP length in flexural retrofitting can make the FRP more effective for concrete repair and strengthening.
- The deflection was reduced in all retrofitted beams and the lowest deflection was observed in the beam strengthened with CFRP in flexure (Sb3C).
- The stiffness of the FRP- retrofitted beams is increased to that of the control beams. This means that, the stiffness of the beam increases with increase in load carrying capacity.
- The study shows that among AFRP, CFRP and GFRP, CFRP wrapped beams has higher strength than others, followed by AFRP.

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